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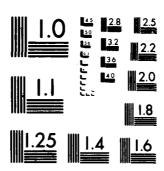
DEFENSE MAPPING AGENCY INTER AMERICAN GEODETIC SURVEY--ETC PHOTOGRAMMETAY SOFTWARE. A PACKAGE FOR EVERYONE. (U)

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## PHOTOGRAMMETRY SOFTWARE A PACKAGE FOR EVERYONE

James R. Hawk Defense Mapping Agency Inter American Geodetic Survey Building 605, Fort Sam Houston, Texas 78234

#### BIOGRAPHICAL SKETCH

James Hawk is a photogrammetrist with the Defense Mapping Agency Inter American Geodetic Survey (DMA IAGS). His responsibilities include providing computer software support to the Cartographic School in the Republic of Panama and to associate Latin American mapping agencies. Mr. Hawk also has over ten years working experience with the Defense Mapping Agency Aerospace Center in St. Louis, Missouri. He received his B.S. degree in mathematics and geography from Indiana State University and his M.S. degree in photogrammetry from Purdue University. Mr. Hawk is a member and past regional officer of A.S.P.

#### ABSTRACT

The development of a photogrammetry software package requirés considerable study. Computer design and capacity, available photogrammetric instruments, analytical methods versus semi-analytical methods, simplicity versus comprehensiveness, user competence, and the desired final products are but a few of the elements to be weighed. An example system is presented for consideration in this paper. This system is being implemented throughout Latin America by the Inter American Geodetic Survey. The package consists of both an analytical and a semi-analytical adjustment program and the accompanying programs which tie the systems together and allow the user to go from relative orientation or comparator measurements to plotter element settings via computer. Included in the discussion is control selection and distribution, pass point selection, program execution, and analysis of results. The sample software package was assembled because it is simplement and yet versatile enough to be adapted to a variety of computers and photogrammetric equipment. It is presented as a successful example of a system design, and it is hoped that this discussion and example will generate new ideas in system development.

#### INTRODUCTION

Photogrammetry may be defined simply as a method of extracting information about an object by studying its image. Photographs are the most familiar types of images, but devices sensitive to other parts of the electromagnetic spectrum may also be used. Regardless of the type of sensor, the image must be analyzed before the correct information is obtained. The desired results could be interpretive and require the art of photo-interpretation to identify and evaluate the image. However, the desired results could be quantitative and require measurements and mathematics to arrive at the desired goal.

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The advent of the computer and its adoption to photogrammetric problems has greatly enhanced this science. Even in photo-interpretation the computer is used to recognize patterns in even the nonvisible region of the spectrum. Quantitative methods have benefited immensely from computer utilization. It is now possible to solve thousands of simultaneous equations in only a short time.

The computer software discussed in this paper was designed to help obtain quantitative results from aerial photography. The results are used to obtain dimensions and precise positions. The desire for increased accuracy has led to the development of precise equipment, systematic procedures, and rigorous computer adjustments. The most common application of the resultant information is in the preparation of topographic maps and charts of the earth's surface. Almost all of the national mapping programs in the Western Hemisphere are performed by photogrammetry.

The goal of the software system developed by IAGS is to encompass the needs of all the associate agencies. It is not burdened with copyright laws and therefore allows everyone free access. It maintains simplicity and yet allows us to obtain meaningful results from a wide variety of equipment. Technical advancement in computer hardware, photogrammetric instruments, and mathematical formulation will not allow any software system to rest. Our programs are under constant change, and we are always looking for something better. The package, however, has been installed and is being successfully used in the United States and in several Latin American countries.

#### SYSTEM DESIGN

The design of any photogrammetric software package must begin with the adjustment program. It is the largest and most complex part of the system, and the other programs are built around it. The wide variety of instrumentation, technology, and requirements found in the hemisphere preclude the use of a single program to accomplish this task. Thus both semi-analytical and fully analytical approaches were taken. Both applications produce accuracy and efficiency sufficient to fulfill our mapping requirements.

The purpose of the adjustment program is to enhance the ground control network that currently exists through field surveying, doppler positioning, or other techniques. Each model used in a stereoplotter requires at least two horizontal control points for scaling and three vertical control points for leveling. It would not be economical to establish all this control by ground methods. Adjustment programs must also provide a method of detecting blunders in the observations, evenly distribute small systematic errors, and determine estimates of precision for the results.

After the adjustment programs were selected, a series of auxiliary programs were acquired and written to blend the two main programs into a versatile smooth-flowing system.

These programs allow us to check observations for blunders, check data dimensions so as not to exceed the adjustment parameters, produce independent model coordinators from two dimensional comparator measurements, mathematically compute projection centers, determine stereoplotter orientation settings after the adjustment, and provide other useful information as will be discussed later in this paper.

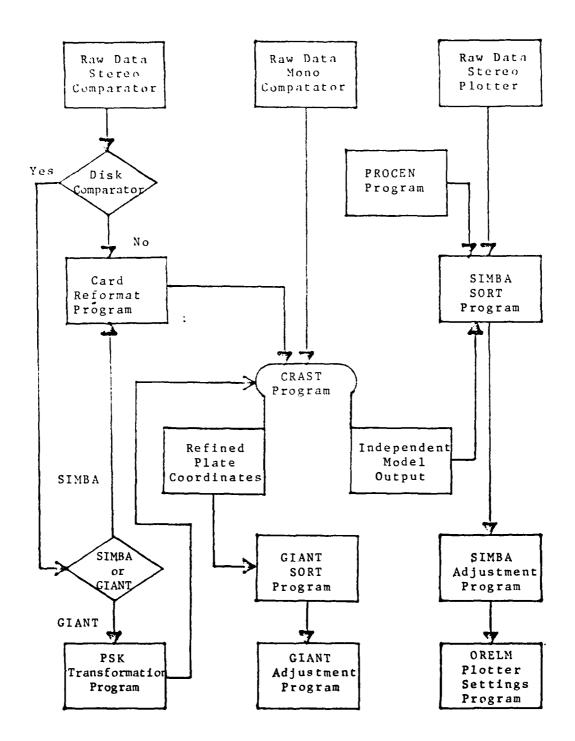
A depiction of the overall system flow is shown in Figure 1. Raw data may enter the system from three different sources, be channeled in various directions and adjusted as desired.

#### THE SIMBA ADJUSTMENT SYSTEM

The Simultaneous Block Adjustment of Models (SIMBA) program was written by Randle W. Olsen of the U.S. Geological Survey in 1973. It has been adapted for use by the U.S. Forest Service and DMA IAGS. It is written in FORTRAN IV and is used to simultaneously adjust independent model units to each other and to ground control. The three-dimensional data may be generated on a stereoplotter or on a comparator providing the CRAST (Coordinate Refinement-Analytical Strip Triangulation) or a similar program is used to derive the projection centers and the third dimension. The models are adjusted by an interactive series of planimetry-height solutions. Four-parameter linear transformations are used in the horizontal solution and three-parameter linear transfromations are used in the vertical solution. This separation economizes computer time and storage without a significant compromise in accuracy as compared to more traditional sevenparamater adjustments.

#### Program features include:

- 1. Input arranged in model units with a separate control deck.
  - 2. Internal sorting of control and tie points.
- 3. In-core solution of banded normal equations to minimize computer time.
  - 4. Separate weighting for ground control and model ties.
- 5. Printed output arranged in model units with corresponding residuals.
- 6. A listing of test point coordinates and residuals which were withheld from the solution.
- 7. A root mean square (RMS) error summary of held control, tie points, and unheld control.
- 8. Absolute orientation values to include common tilt, common tip, the airbase, and exposure station elevation difference for each model.



Special attention should be given to the weighting system as it affords us the opportunity to separate errors resulting from poor control from errors due to relative orientation or bad model tie points. The equation for the expression of the weight ratio is:

$$W = \frac{2 \cdot \operatorname{Sm}^2}{\operatorname{Sm}^2 + \operatorname{Sc}^2}$$

Where Sm = estimated unit coordinate standard deviation of the model points

Sc = estimated standard deviation of each control
 point

One can easily see that the value of "W" will range from zero to two. A value of one will allow equal weight to be placed on the control value and the model ties. A value close to two will constrain the control and allow the models freedom to move. Likewise a value close to zero will constrain the models and allow the control to shift.

A minimum of two horizontal control points are required for the planimetric adjustment, and three vertical control points are required for the height adjustment. If no additional control is used, there will be an absolute solution and no errors will be propagated by a least squares adjustment. Thus if only minimum control is used and constrained while the model ties are given freedom to move, the propagated error will result from poor model fit, and the tie point residuals will be exaggerated. Actual errors present in the constrained control are unimportant because they are only being used to scale and level the block solution, and we are only checking tie point residuals.

Once a good model tie solution is achieved, the full control network is used, but the weight is changed to constrain the models and give freedom of movement to the control. The control residuals thus become exaggerated, and control values incompatible with the relative model solution can quickly be noted. After all blunders have been eliminated, equal weighting is generally used in the final solution unless extenuating circumstances dictate otherwise.

The computer memory necessary to execute this adjustment is largely dependent upon the size of the normal equation coefficient matrix. If the independent models are properly aligned in the program, this matrix becomes banded along the diagonal leaving only zeros outside the band. To conserve storage, only the elements within the band above the main diagonal are stored. They are arranged in a rectangular array called "Q" in the program and are dimensioned 4 x (the bandwidth) by 4 x (the number of models). Correct sequencing of the models is therefore critical to stay within the allotted bandwidth. In a rectangular block, the bandwidth can roughly equal the number of models in the longest strip plus two. The normal equations are solved by the Gaussian elimination procedure using routines suited for

symmetric and banded equations. The banded routine conserves computer core and significantly reduces execution time.

It is always difficult to recommend a control point pattern because there are so many variables, the most important of which is quality. Also, it has been my belief that a good photogrammetrist will attempt to secure as much control information as possible, and his only limiting factor will be the man in charge of expenditures. At the risk of violating this rule, a control pattern which should satisfy most needs is recommended.

It is essential to have horizontal control in the extreme corners of the block. Any deviation from this will cause warping in the corners becoming progressively worse as the control point strays from its corner position. Other than the corners, SIMBA seems to hold horizontal position rather well. Additional horizontal control should be spaced around the edges of the block at six to eight model intervals.

Vertical control is a different matter. The adjustment acts as a blanket tacked down at the corners with a fan blowing air under it. It bulges in the middle, and when we tack down the middle, we develop two smaller bulges on each side. This progression continues geometrically until the bulges are reduced in size to an acceptable level. The general recommendation is to have a vertical control pattern at three to four photo intervals throughout the block. A sample control point pattern is shown in Figure 2.

Pass and model point selection is a matter of personal custom, but the author has developed a system which lends itself well to both the adjustment mathematics and production flow. Starting with one of the border strips in the block, pick four points in a line in all the trilap areas and at the two ends. Insure that the two points closest to the adjacent strip fall in the sidelap area and are transferred. Transfer these tie points to the second strip and send the first strip to the comparator or stereoplotter to be measured. Pick points on the second strip as before. If transferred points from the first strip happen to fall in the trilap or necessary end positions of the second strip, use them. If not, continue point selection as on strip one. It is not necessary to transfer any of the newly selected points back to the first strip. Continue in this manner until all points are selected. This method will produce from eight to ten pass points per model and is illustrated in Figure 3.

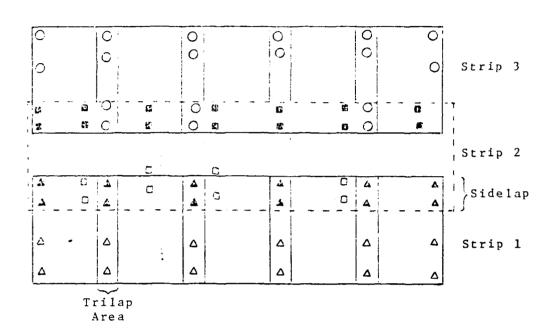
Three versions of SIMBA are currently available at DMA IAGS Headquarters in San Antonio, Texas. These versions are the standards which are then adapted to the computer and desires of individual users. The first version is the most common and has parameters of 240 models, 2200 unique points, 2500 total points, 150 control points, and a bandwidth of 25. It uses 526K bytes of storage on an IBM 370 computer. The second version of SIMBA breaks the program down into three

CONTROL POINT SELECTION

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- ☐ Horizontal Control
- △ Vertical Control

### PASS AND TIE POINT SELECTION



 $\Delta$  Points selected on Strip 1

. . .

- ☐ Points selected on Strip 2
- O Points selected on Strip 3

Between-strip tie points are shaded.

parts which are executed separately. The first part reads the input data, organizes it, and builds a file. The second part reads that file, performs the necessary mathematics, and writes the massaged data back onto the file. The third part reads the file and prints out the desired results. This three part version is a computer core saver but sacrifices time and simplicity. Dimensioned similar to the original program, the three part version will save about 75K bytes of storage. The third version we have is another three part program with the first and third parts remaining as before. The difference in part two is that a direct access disk file is used to write and then read each row and column of the normal equation coefficient matrix. All of this reading and writing takes time but an additional reduction of 125K bytes of computer memory is realized. This version is impractical unless the computer memory is limited and computer time is of no concern.

A sample SIMBA output is shown in Appendix A.

The SIMBA adjustment requires the use of perspective centers to strengthen the vertical solution. If a stereoplotter is used for relative orientation, the coordinates for the perspective centers may be computed using either one of two available programs. The ideal solution for determining the camera perspective center is by space resection using a reseau grid. The IAGS version of John McLaurin's Perspective Center Determination Program (U. S. Geological Survey, 1969) does precisely this. A grid of known precision, usually engraved on a glass plate, is projected through a stereoplotter projector. The coordinates of from three to fifty grid intersections are then measured in model space. The bundle of rays passing through the grid intersections on the calibrated plate and the bundle extending to the same measured intersections in model space originate from the same point--the perspective center. If the latter bundle of rays is fitted to the other bundle in a least squares space-resection adjustment, accurate coordinates for the perspective center can be determined. Printed output includes:

- 1. Title.
- 2. Input data to include point numbers, number of readings on each point and grid coordinates.
  - 3. Number of points used and computed principle distance.
- 4. Mean projected grid coordinates and standard deviations.
  - 5. Parameter values after each iteration.
- 6. Residuals of the projected points in their coordinate system.
  - 7. Variance-covariance matrix.
  - 8. Standard errors.

Although this program is quite accurate and produces a wealth of information, it is time consuming to use. This, coupled with the unavailability of calibrated grid plates and stereoplotter variety encouraged IAGS to develop a Simplified version. For instruments such as the Wild A-7, A-8, A-9 and A-10, the perspective centers do not vary with the relative orientation of the model since the axis of rotation for omega and phi intersect the projection cardan. The perspective centers change only if the instrument base component is changed. Thus the coordinate determinations need be made only once per strip or, in special cases, per block. However, for optical instruments such as the Zeiss C-8 and Wild B-8, the projection center varies with the relative orientation of each model since the axes of rotation for omega and phi do not intersect the projection cardan. If the perspective center is to be determined after each model relative orientation, a quick method is needed.

Instead of a reseau grid, the engraved lines and crosses on the plate carriers are measured and substituted for a grid. These intersections can be quickly measured after relative orientation of each independent model. The instrument z and the numbering sequence must remain constant for each set of recordings. The program is designed to compute perspective centers of the left and right cameras alternately. A maximum of ten points may be used, but six points are customary. The program output is limited to the x, y, and z coordinates of each projection center, but this is sufficient input for the SIMBA adjustments. An example program execution is shown in Appendix B.

One's first experience with a large adjustment program can be frustrating if two unique points have the same point number. The block tends to fold in upon itself causing large residuals and an unsatisfactory solution. Another frequent error is the exceeding of the bandwidth parameter. This is a result of long flight lines or incorrect model sequencing in the program. SIMBA is unpredictable when this occurs and may diverge instead of reaching a solution. These and various other errors present in the data can be difficult to locate. Also, it is expensive to run large adjustments if errors are inherent in the data. For these reasons the author wrote the SIMBA SORT program (SISRT). It uses the exact same data deck (or file) as SIMBA and is quick and inexpensive to execute. The main program function is to do a point sort and list the location by strip and model of each unique point. It also computes and lists the bandwidth of each point. This allows the user to insure the SIMBA bandwidth is not violated, or if it is, which point is the culprit. A complete data deck/file listing is provided. Sample program output may be found in Appendix C.

The Numerical Model Orientation Program (ORELM) was written by Randle Olsen of the U.S. Geological Survey in 1976. The purpose of the program is to use the omega and phi elements from relative orientation, the desired model scale, and the common omega, phi, airbase, and bz for saca noted is amounted by HMSA to produce the input absorlate effection that a variety of attroophorter. ORELY can be used even if the photos are reasoned on a compartor as the GRASE program will compute the proper input orientation ongles. Specific plotters which can be oriented are the Bulblek (ER-S5), Kelsh, Wild B-3, Kern PG-2 and the U.La A-3/PPOS. We are currently working to incorporate the Beiss C-8 into this list. The projection plotters require graduated dials or auxiliary equipment to utilize this data. The use of the program eliminates the need to reorient, scale, and level before plotting. Sample output may be found in Appendix D.

#### THE GIART ADJUSTMENT SYSTEM

In this analytical photogrammetric system, the physical reconstruction of model geometry is replaced with mathematics. The mathematical model is constructed to represent the relationship letween the photographic images and the object space. The images are measured on a comparator and numerical methods are used to produce the position and orientation of the exposure station and the image coordinates in object space.

Analytical and semi-analytical methods are both capable of producing accurate results. Therefore, accuracy is not the justification for the extra expense of a comparator and the analytical programs. However, there are inherent advantages in an analytical system. First of all, it is much easier to bridge water or snow fields. One does not have to worry about trying to remove paralax from a model that is half water. Secondly, a large variety of photography and cameras can be accommodated. Panoramic photography, for example, does not have a simple central projection and special cameras on high flying aircraft may have a very long focal length which is impossible to duplicate on a stereo comparator. Thirdly, analytical methods permit the input of auxiliary sensors. Such sensors include stabilization systems, profile recorders, altimeters and astronomical observations. This additional information may be incorporated directly into the solution and weighed according to reliability. It should be noted that some semi-analytical programs now have this ability.

Analytical photogrammetry offers great potential. It can incorporate various distortions and deformations which are impossible to duplicate in stereoplotters. The proponents of analytical methods claim they should be able to obtain more accurate results, but in practice this has not proven to be true. The complex mathematics and equipment costs have frightened away many potential practitioners. IAGS does not recommend one method over the other, but instead has incorporated both methods into the software package.

When photogrammetric images are measured on a monoscopic or stereoscopic comparator, two dimensional coordinates are produced. The CRAST program refines these coordinates and produces either GIANT input or independent model coordinates for input into SIMBA. Mathematical refinement of the comparator measurements is necessary because the model space was not physically reconstructed. It is mathematically

reconstructed, so we need to knew additional information to accurately duplicate what took place when the photos were taken. The first three items on the following list are required for program execution. The remaining items will be used if they are known and input.

- 1. Camera Focal Length
- 2. Approximate Flying Height
- 3. Schut Refraction Coefficient
- 4. Comparator Scale Corrections in x and y
- 5. Comparator Non-Orthogonality
- 6. Initial Airbase at Photoscale
- 7. Radial Lens Distortion Corrections
- 8. Principle Point Variation from Photo Center
- 9. -Calibrated Fiducial Coordinates

Depending on the information input and the desires of the user, the CRAST program will correct the input coordinates for the following distortions:

- 1. Film Distortion
- 2. Earth Curvature
- 3. Light Refraction
- 4. Comparator Calibration Corrections
- 5. Radial Lens Distortion

If the GIANT option is used, CRAST output will consist of a fit of fiducials to camera data, the refined plate coordinate listing and punched card output in the GIANT format or an optional format. If the SIMBA option is used, output will consist of independent model coordinates, y-parallax, model tie residuals and card output in the SIMBA or an optional format.

The reason for the use of this particular program is its adaptability to the lack of information which frequently is found to be the case in Latin America. For example, the program will accept the absence of calibrated fiducials and even the absence of fiducial observations. CRAST was originally written by Randle Olsen in 1973 and has been revised several times, including a revision by IAGS in 1981. Sample program output can be found in Appendix E.

The General Integrated Analytical Triangulation Program (GIANT) is designed to perform a least squares adjustment of a block of frame photographs. It was written by Atef A. Elassal in 1976 and has been adapted for use by the U. S. Geological Survey, the U. S. Forest Service and IAGS. Many variations of the program now exist and at this writing, it is currently undergoing yet another revision at IAGS.

GIANT will analytically solve for the ground coordinates of image points measured on two or more photographs. It also solves for each camera station position and orientation. Only uncorrelated observations are acceptable and may be individually weighed to reflect known precision. This system allows the mixture of horizontal and vertical control of varying accuracies. It also allows the use of known camera station parameters. The program propagates error estimates through the solution, computes the a posteriori estimate of the variance of unit weight and has an option for listing the variance-covariance matrix and standard deviation of the output parameters. These indicators are useful estimates of the solution accuracy.

An initial approximation is required of each camera station position and orientation and of each unique image point. The approximations of the image points are computed internally and gross approximations are acceptable for the camera parameters.

Program dimensions require that no strip exceeds 20 photographs or that there are fewer than ten strips in the adjustment. IAGS currently has two versions of GIANT on file with the difference being in the dimensions. The principle version has the following parameters:

- 1. 460 photographs
- 2. 450 ground control points
- 3. 9509 unique ground points
- 4. Each point may appear on up to 10 photographs.
- 5. Object space is expressed in space rectangular coordinates only.

The small version is similar with the dimensions reduced to 150 photographs and 150 ground control points. The large version requires 476K bytes of computer memory on the IBM 370 computer. The core capacity is reduced to 282K bytes in the smaller version.

Experience has shown that several different types of executions prove helpful in data analyzation. The idea is similar to that used in SIMBA. One must try to separate the affect of different observations in order to locate blunders. The recommended execution sequence is as follows:

- 1. Use the intersection-only option with no ground control. Check for large blunders.
- 2. Use the triangulation option with no ground control. Check for smaller errors in point marking and measuring, especially in the tie points.
- 3. Use the triangulation option with ground control. Weigh the image measurements tightly and allow the ground control to move. Check for ground control errors and update the exposure station parameters.
- 4. Use the triangulation option with full ground control. Weigh the solution realistically; check all residuals.
- 5. This is the final execution. Use the triangulation option, full ground control, realistic weights, and the latest estimates of the exposure station parameters. Use the error propagation option.

There are several things to watch for during the program executions. Most must be learned through experience. One important factor to remember is the bandwidth limitation. It may be controlled by careful ordering of the frame position and attitude cards. One can stack the photos in any order. The trick is to keep the frames with common points as close together as possible. The pass and control point selection procedures are the same as for SIMBA (see Figures 2 and 3). It should be noted that GIANT will not distort block corners as severely as SIMBA, so corner positioning is not as critical.

The output listing includes:

- Camera station parameters with residuals.
- 2. Plate coordinates with overall standard deviation of  $\boldsymbol{x}$  and  $\boldsymbol{y}$  per frame.
  - 3. Ground control coordinate listing with residuals.
  - 4. Camera station corrections per iteration.
- 5. Triangulated residuals per frame for all ground points.
- 6. Triangulated camera station position and orientation residuals.
  - 7. Triangulated ground point coordinates.
  - 8. Applied ground control corrections.
- 9. Variance-covariance matrix and standard deviation for the camera station parameters and ground control.

Excerpts from a typical program execution may be found in Appendix F.

As with SIMBA, problems with mispunched or misnumbered data points and errors in the organization of the data deck can cause time delays and needless computer expense. A data clean-up program was written by the author in 1980 to avoid such problems. This program (GISRT) is similar to the SIMBA Sort program in that it uses the exact same input setup as GIANT and does a point by point analyzation. The output consists of the following:

- 1. Data deck listing.
- 2. Frame position and attitude card check.
- 3. A frame count by strip.
- 4. A point by point listing to include all locations encountered and bandwidth computation.

A sample of the program output may be found in Appendix G.

#### SUMMARY

The programs presented here are not state-of-the-art technology. They do not have the most rigorous solutions nor the most sophisticated computer manipulations. What they do have is a good combination of flexibility, mathematical rigor and computer independence. The overall package can and will be improved. It is hoped that a new adjustment program written especially for mini-computers can be incorporated into the system. The GIANT program is under revision to make it more user oriented and to add additional options. One such option would allow output in geographics. The package is presented as a model for discussion. Its greatest testimonial is that it is being extensively used and has proven successful.

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APPENDIX A

SAMPLE OUTPUT

FROM SIMBA

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45	196	342554. 32	30494411	5116.30	71.0	CH.U	*0*0		
5	17.7	33-44-1-15	3702219 34	367.46	:				
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ş	[417	347651866	123/24/201	20, 20,	21.0	70.2	20.0		
2	1353	347-46-(3	47.47.44	2,30		2 - 2 -	)		
96	1441	342734.41	31/11/1320-2	10.121	14.0	-1, 03	0.01		
32	714	342435.24	3792311.14	425.49	12.1-	-0.0-	0.42	riuk/vita	
ç	4474	340474.15	3047404.14	13.75.	Cu.0-	-0.21	77.0-	コロドノドス	
ç	147.6	340275493	3691413034	231.30	U 1	V - 36	-04.40	חטא/צנא	
65	****	349277.04	35.04077.74	195.30		-6.17	1.53	X 2 X 7 X D H	
46	YASAB	340436.HZ	3700155.24	310.18	1.65	-10.14	1.53	HUM/VEH	
3,	<u> </u>	342344.32	3545497611	45.34	- ^1.c-		10.01		
55	145	345403.44	41.142444	51,55.42	7, 1	1.25	0.26	,	
3	1441	3+2/35.41	3701436.42	HO-174	10.0-	0.03	-0.01		
ĩ,	1 163	342145.78	3547403.33	(19.00)	0.11	0.35	-0.24		!
5	15">	3441143.41	35.21.6314.34	733.37	1. JH	12.0	0.11		
	1 c'x 1	345704.78	3701886-02	331.70	0.16	در. ن. در. ن.	-0- 0-10 04-0		
3	135	345414.44	1449207.79	2162.96	10.43	-1.25	-0.26		
33	40.1	344345.05	3549155.54	5131.49	72.0	0.50	14.0-		
*	1221	345504.14	3/01842.02	116.70	21.0-	40.0	-0.45		
3	1953	364797.35	46.56.51.44	2.14.0.4	در. ٥٠	0.33	0.13		
3	1902	347324.67	3546466.10	40.146	1).(1	-0.15	-0.24		
3,	1 /4 9	344101-44	30,444,9004	10.00	0.10	0.4]	-0.23		
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4	[402	341543.74	3447453.51	4.5.54.5	9 n.e	12.0-	12.0		
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•	44.44	347643.32	3695877.31	いく・トレン	د <b>ن.</b> باء	-0.75	0.50	カライント	
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5	1.46.5	346175.74	3097403+31	10.000	57.0	-0.25	-0.15		
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ς	2043	341915.48	3442445.25	200.01	-11 - 40	U.3H	-0.38		
1	4.15.5	1497 70 401	20125125	752.31	1.7.0	V.11	-0.10		
5	1502	344083.41	3597251•34	45.04.9	-B+64	-0.01	0.39		
ć	492	344415.43	3992024 • 15	7161.24	-0.37	5 X • 0 =	-0.3/		
٥	2.16	347334.27	3245044 10	5121.33	14.0	1.03	0.15		
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q	2371	34440247	3593354.40	267440	-0.13	15.42-	0.4.0		
9	といっろ	344-70.01	3742574.58	255.31	<b>71.</b> 0	درۍ	-0.32		
1	2,53	347247.15	02,1012,42	750.67	U • 1 0	-4.V-	-0.17		
v	70745	347646.10	3594] 74.49	00.675	a / • □	14.0	-0-17	おいまくとにお	
٩	402	14/12/24/27	36:16:120:12	211.99	-2.13	9.03	45.0		
•	4647	347303.54	3295/83.25	04° 542	10.0-	-0.20	20.0	おしよくとれる	

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<u> </u>	54.02	35441752	8042401-31	761.44	-1.15	70.4	6.0	
2	633	355473.44	10.4190445	5111.00	10.11-	£.1-	0.40	
4	< 1¢	35 Hilland Pr	12-2010205	5220.41	/1.1.	1.02	-()-31	
25	1882	335 477 64	コナ・ナフー・ナジャ	40.105	C, T = C	20.0-	00.0-	
2	7331	35-111.17	プラ・ド ピナルピルト	711.11	11.0	112.01	10.1	
32	6363	350050	3534/55.35	CF - F / 7	C1.1.	-0-11	11.0-	
35	1787	35:214.18	3743143.64	254.33	-0.10	47.0	41.0-	
2	73.449	36.11.46.16	3543444.53	( 4 . 1 (1 /	01.0	٠١٠)	-0-1/	HUM/VEH
3	21.13	3745'14.46	3542737.75	704.33	11 7 • ( 1	-0.27	0.26	
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3.1	162	335444.12	3538684 34	5205.48				
ī	c 37.1	350219.18	3543140.44	204.33	5.0	-0-15	00.00	
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15	6313	352416.62	1044 740.12	サイ・ノテス				
15	6.311	326417.64	3443145.31	د1. دم <sup>ر</sup>				
ış	6113	361114.13	3642344.18	802.09	77 • U=	-0-44	29.0	
15	73104	350 145.48	3692240+35	1,000442	3.40	-8.16	0.54	HUK/VEM

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۲۰۶۱-H7 OMEGARINSTHUMENT GHADS DOWNWAND TOWARD HACK PHIS INSTRUMENT GRADS DUNNWARD TOLAND MIGHT ALUMAR 3027.2 2274.3 2774.3 27 2430.4 2430.4 2442.1 3051.1 2741.1 3077.1 2.745 2.745 2.745 1.334 0.607 -0.755 17 F 13 A -1.441 -0.554 -0.950 MODEL 242425

APPENDIX B

SAMPLE OUTPUT

FROM PROCEN

PERSPECTIVE CENTER DETERMINATION	ETE 2MINATION			1			
NUMBER OF POINTS TO BE USED	HE USED 6						
FOCAL LENGTH IN MILLIMETENS		153.00					
РНОТО САНЧТАGE 509 1 509 2	177.24	501.09	407.13				
					1		

APPENDIX C

SAMPLE OUTPUT

FROM SISRT

GROUND CONTROL	INCLISTIN							
7 2 2	34-14 37.44	47 -> 311 - 1 +	177.40	4.14.7.41.7	 	.0.	()	
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4147	-cheathrai	- in Henry 1	4.1.2	THIM / WEE -	h	, h		
¥ ( ).4	34047045	30-14/4.44	21/10	411/11	÷ =	÷	0.	
HLF-47	Statuster.			- MINTAL M.				
1178A	3404] 5.54	37.346.47.1	774.30	711/2011	• 0	•		
	44.4.4.0at	- Breed Santa	2445	11.17.41.1				
1 4 1 1 V	34-176-4-52	4741124.33	45.40	21.10/01/11	=	0.	0.	
417	41.45.34.44					1	110	
74.6.1	347347.45	10.4022014	ž	H.M.Zyři	• 0	• o	tt.■	
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7:4762	714,11500	1247117	4004	11.77.11.1		17.	1 v	
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73333	3472241	32241/101/	215.29	- 4.47 ATE		46		
/# I + 1-3	347659.83	14.11.47	256.03	7 30 / 41 1	9	• •	• =	•
731123	34754243	45 Land	451.54	- MUNK / NEW		u de	U.	
* 1	353171.41	4702114.04	45¢+4¢	HUM/UF1	•	• ;•	•=	
4.47	11-1-1-1	1100473213	16.50	4.14/41.4	47,	110	* 73	
2£3r	47391H.HJ	11:0453.13	174.45	301/101	0	• 0	• 10	
4637	-141411	15.42.46.50.1.3	312.51	30E 24't i		- П -	ή.	
1.6.4	127741.23	35684F1 3.31	5.7.5	MILE / Jrv.	• 77 .	• 5	• 2	
4:14/	Seat Ites	15':14'14'5'	213.64	31.21.7.41.2		1.	٠,١	
2E H 3	3-371-11	\$1.46.56.5.11.4	コン・ヘエン	417/2014	• 5	٥.	· (1)	
11.47	323/2/443	45,545,471	231,441	11.14/ 11.11	1	a A	• (1	
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13334	350144.15	45.4394.43	224.44	HOH/ 154	• =	•	• 12	
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大田を成立しては、大田の東京の中央 中央 ワイン・シャン

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APPENDIX D

SAMPLE OUTPUT

FROM ORELM

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	NO 837u		AND HACK	UME 64-2	0.727 0.H14	1.561	957-0	0.637	0.324	0.3H7	11.472	016.0-	-0.453	 									
	- 1	Iths	рівнт	OMEGA-1	0.517	157.0	24994	0.217	47.6	0.067	0.036	-1.40×	-1.543						:				
	044	JA444	EMERIES TOWARD	0.45	0 .					0	6	7											
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	HAIIO	MODE1	FL SCALE	E = 1:2650	0.005			
HODEL HACE	BK(rT) Bi	BK(M4)	1HZ-3	1-1Hd	DIALGA-1	Z-1H7	DMEGA-C.	
	4/13	5446	120.17	101.84	99,38	101.41	99427	
	4/13		44.66	24.14	100.73	44.15	100.62	
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APPENDIX E

SAMPLE OUTPUT

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Trave Cadrulmate replantant and State Triansulation alth AMALYLICAL ORIENTATION - USGS - NO. H252 - 3/13, Mouf.4/19 MESTON 184 370	Jolyja Test OF HZDZ. ULSEN 1.C.4. NITH ANAL. OHIENTATION		0.6	Synuk Falc Mabla Lews olsfortion Cowaccifons= MICHOUS at 10 MM Fult MALLS. 601013101515151512498A911.	7-60-11-01= FICIE:11 = 40.00	= (S(C×O) =	1 CODMECTION:= USFUHABITION TO FINUCIALS	inalts (ATCHONS)=	.0 -105999.0 .0 -105999.0 .0 105999.0 .0 106005.0				
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APPENDIX F

SAMPLE OUTPUT

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APPENDIX G

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